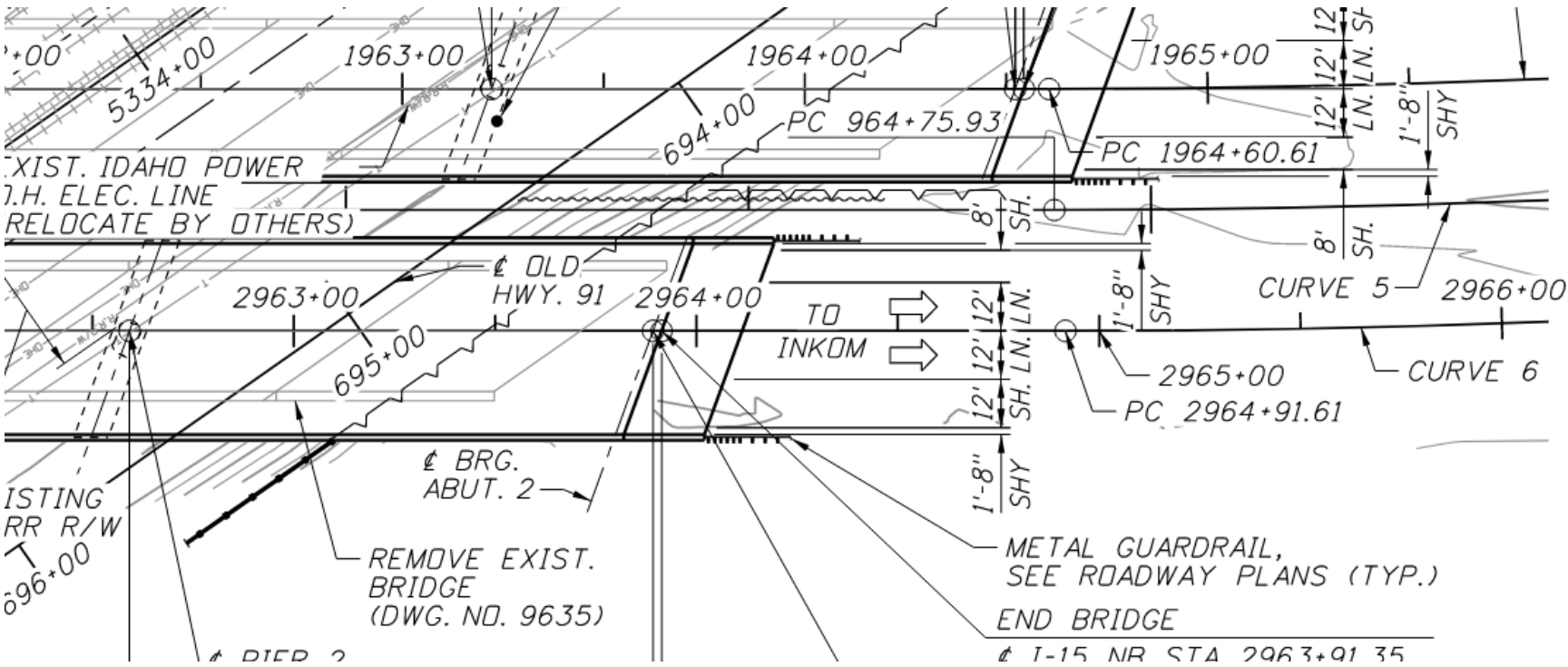


**HDR**



# How I Raised My Inkom

## Inkom UPRR Bridges



Mike Slegers, PE



- 1 General Background
- 2 Goals and Challenges
- 3 Approach and Concepts Investigated
- 4 Design Details
- 5 Lessons Learned
- 6 Q&A



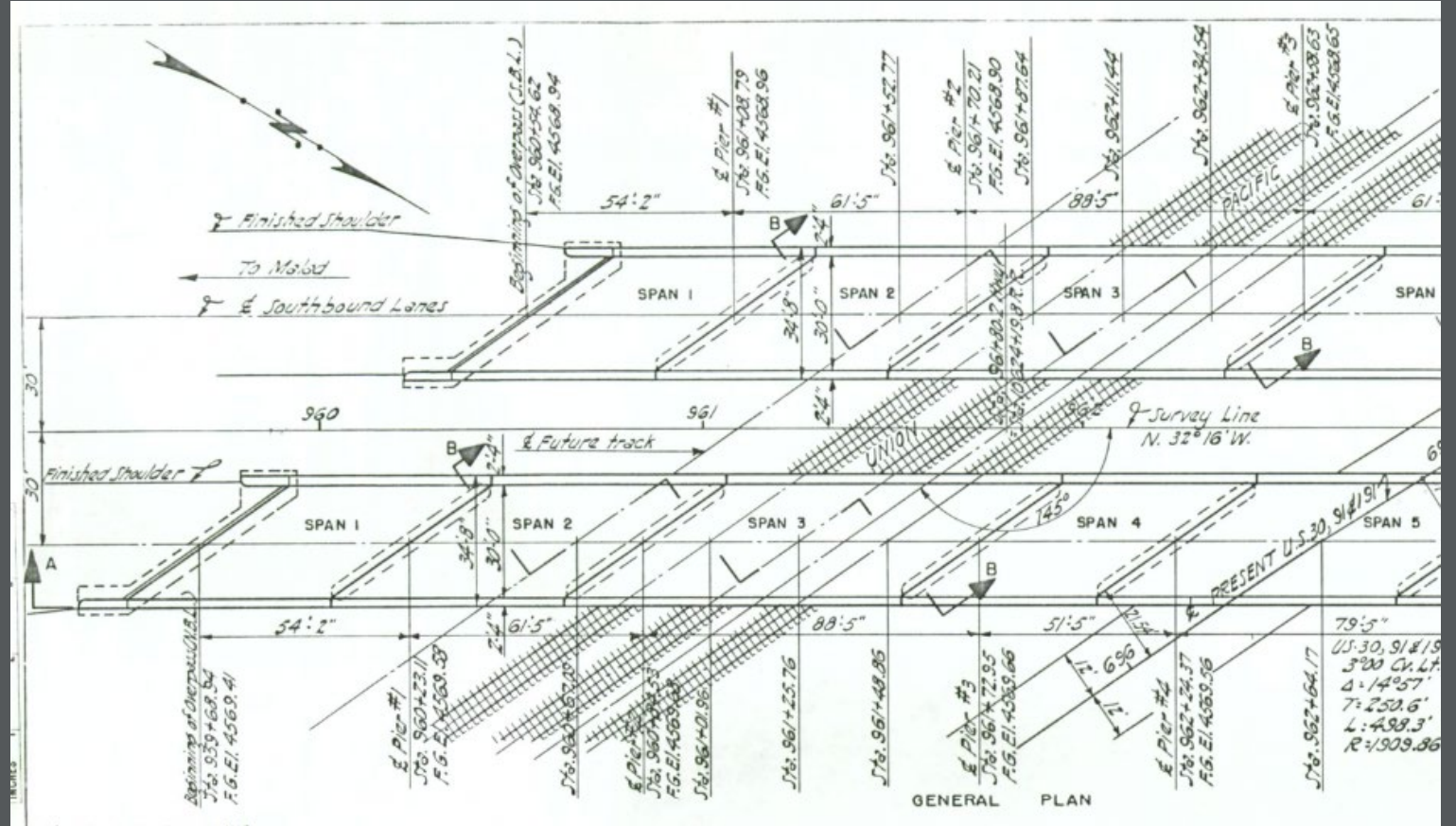
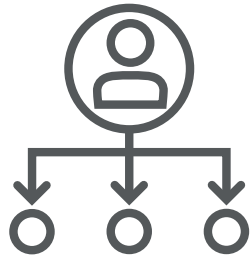


# 1 General Background



# Project Team

- Idaho Transportation Department, District 5 - Client
- HDR – Prime consultant







# Location



# Location







## 2 Goals & Challenges





# Project Need

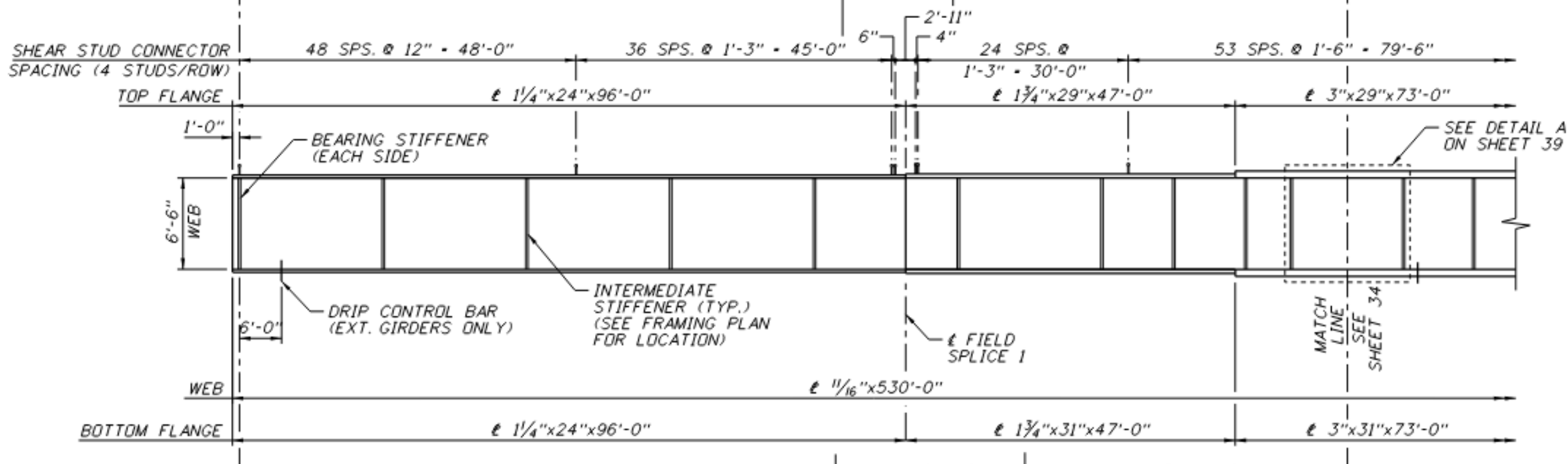


# Challenges

- Skew of railroad tracks
- Vertical clearance of tracks
- UPRR ROW
- Proximity of piers to railroad tracks
- Utilities
- MOT during construction





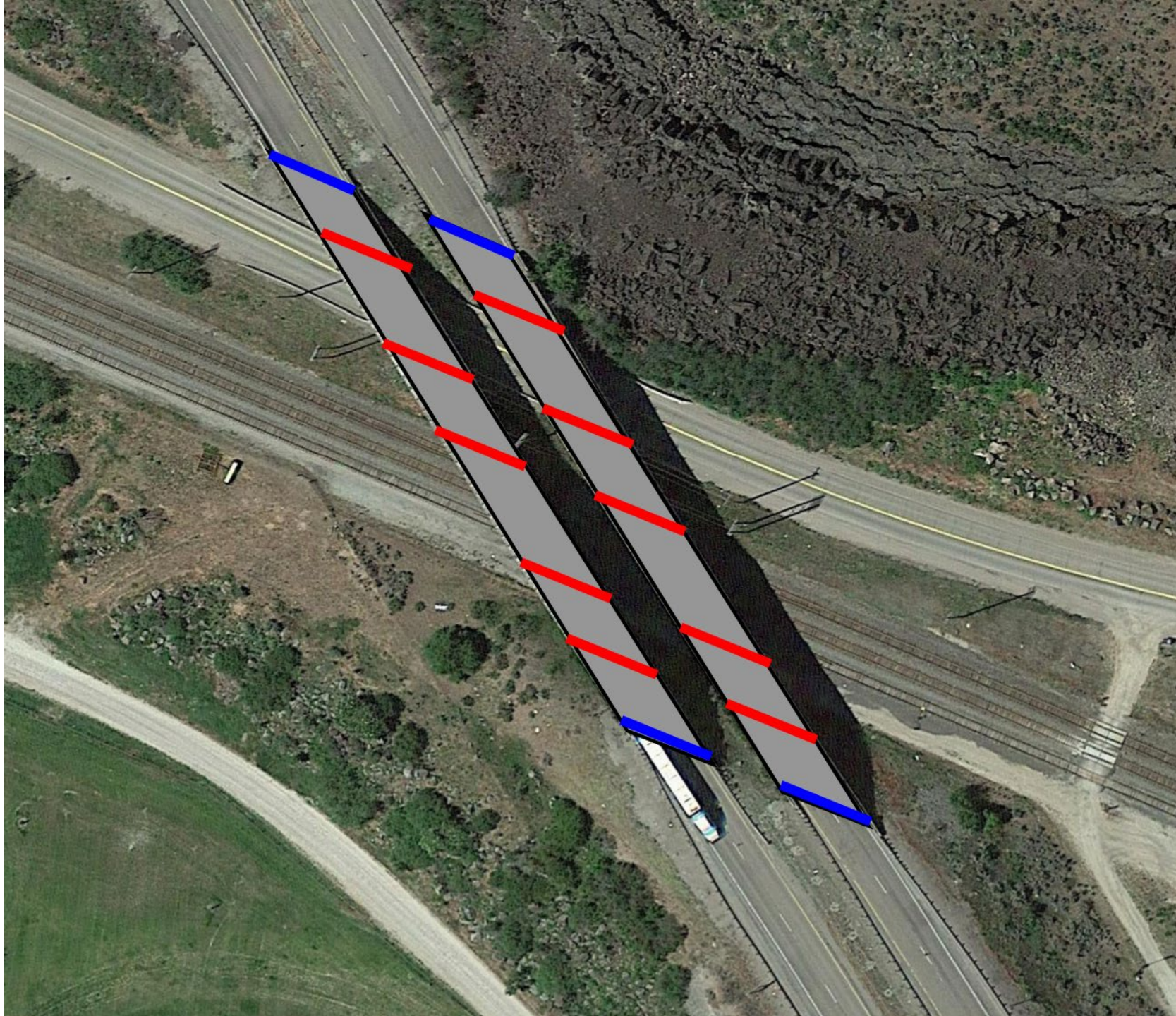


# 3 Approach & Concepts Investigated



# Existing Bridge

- 6 spans
- Concrete piers and girders
- 55 deg skew to tracks
- Low vertical clearance





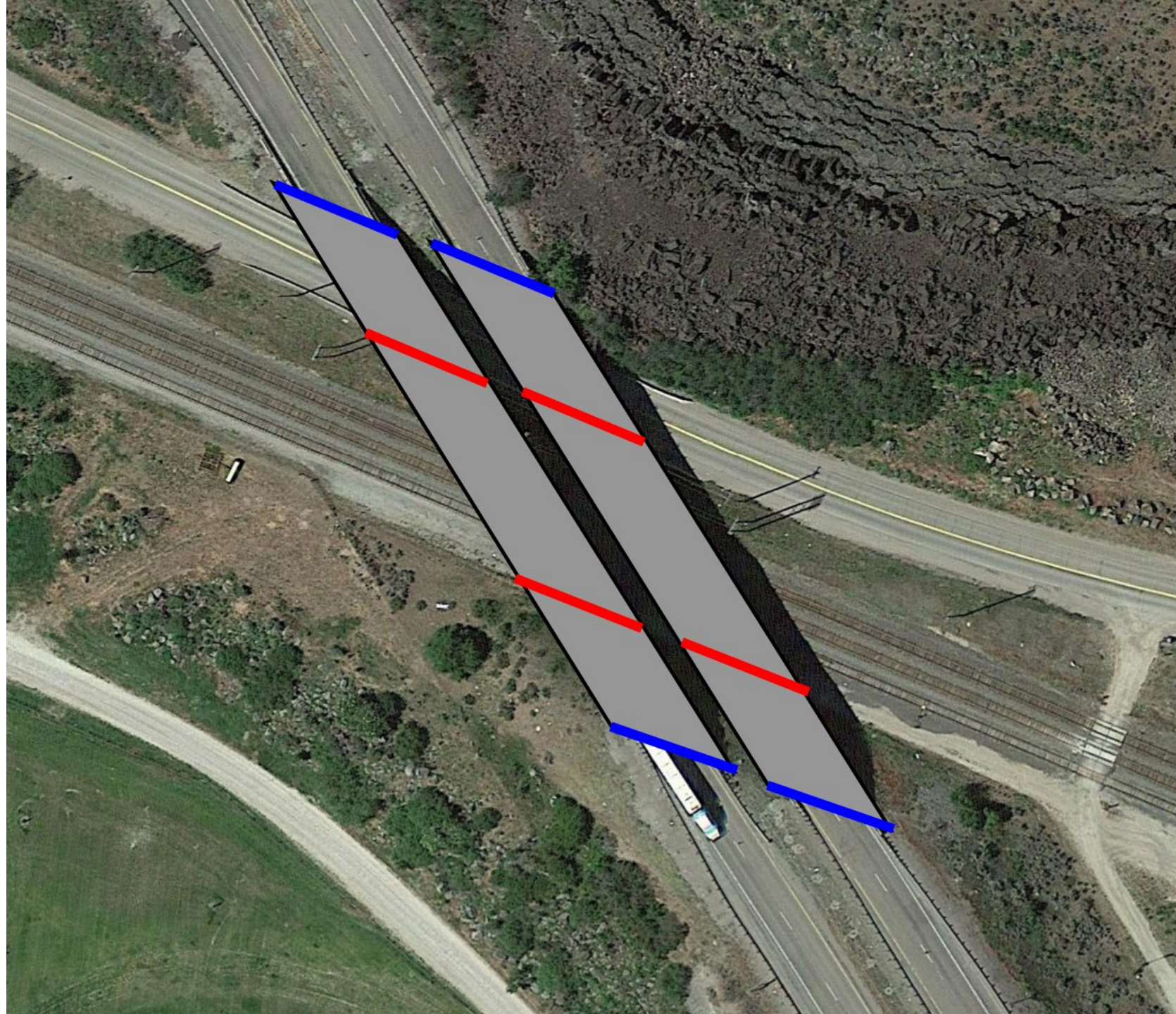
# Type Selection

- OPTION A

- 3-span bridge
- high skew with conventional piers
- PRO
  - Shortest bridge length

- CON

- most expensive (+5%)
- large piers and large footprint in UPRR ROW





# Type Selection

- OPTION B

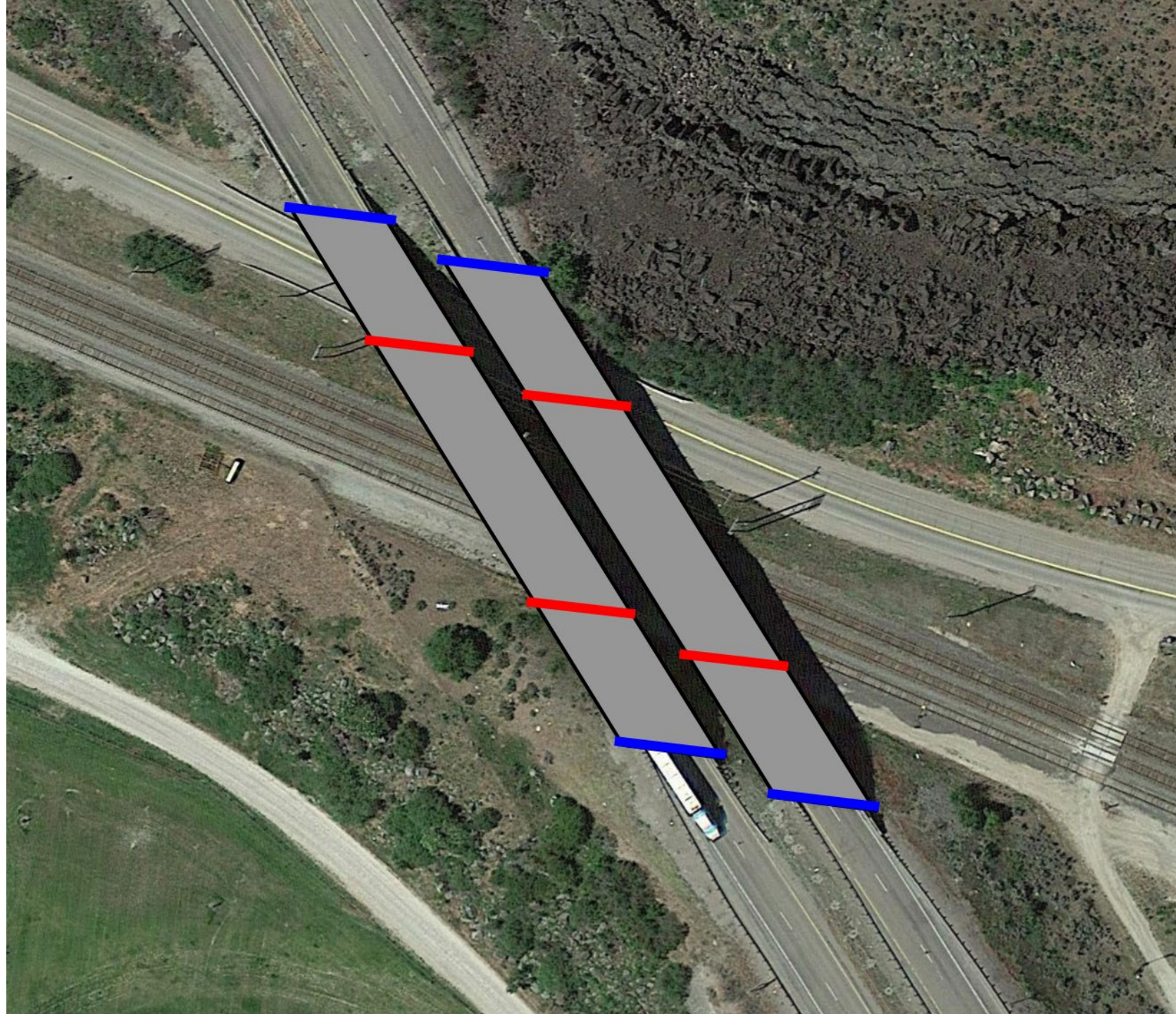
- 3-span bridge
- moderate skew with conventional piers

- PRO

- Moderate skew

- CON

- also expensive (+5%)
- Longest span (255ft)

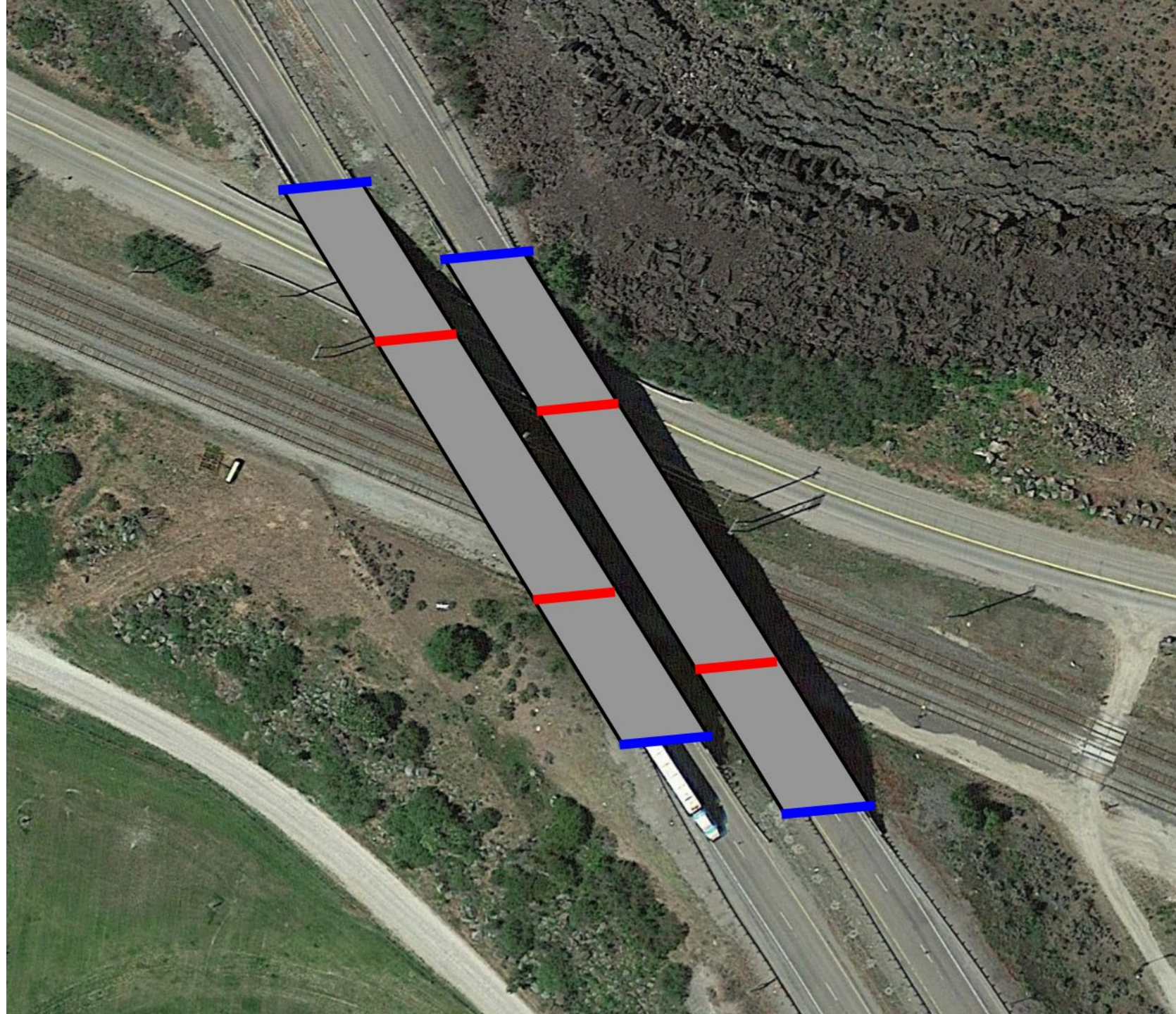




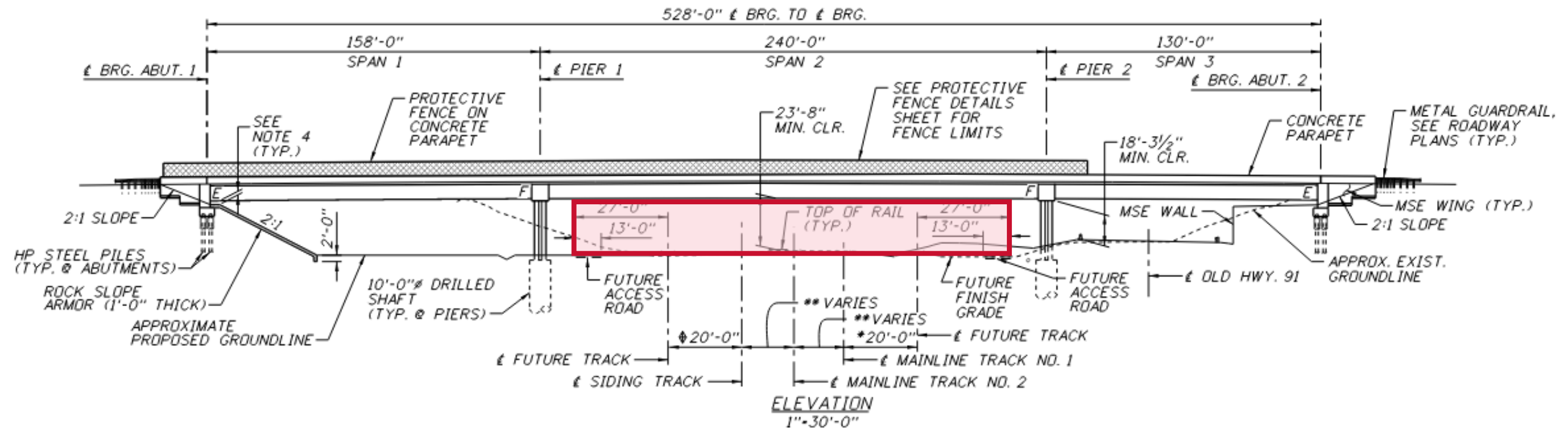
# Type Selection

- OPTION C

- 3-span bridge
- Mild skew
- non-conventional piers
- PRO
  - Shorter main span
  - Smaller pier footprint
- CON
  - Specialized design
  - Longer overall length



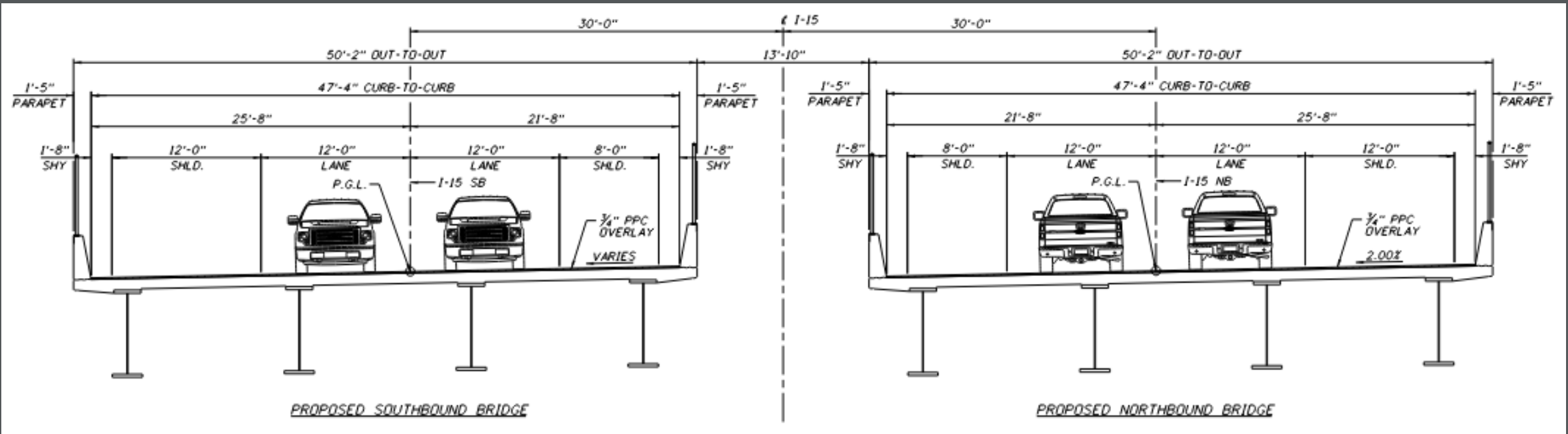




## Type Selection

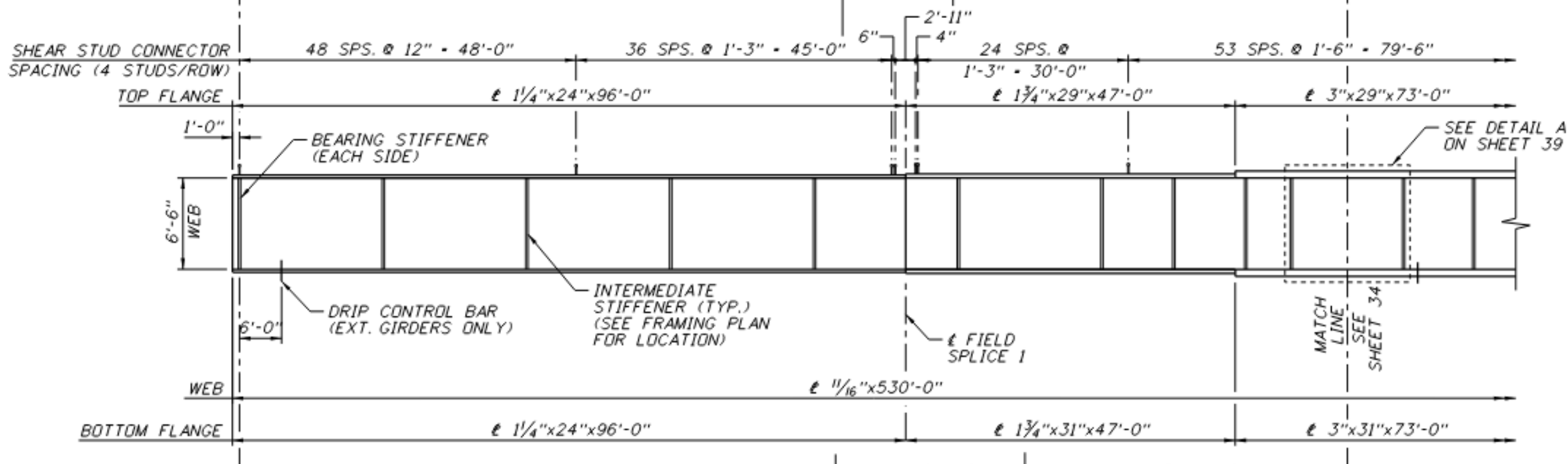
- 3 span steel girder bridge
- 158-240-130 span configuration
- Integral, hammerhead-style piers
- 6.5-ft tall girders; 7.9-ft superstructure
- $23.7' + 7.9' - 18.9' - 4.7' = 8\text{ft}$  GRADE RAISE





# Typical Section



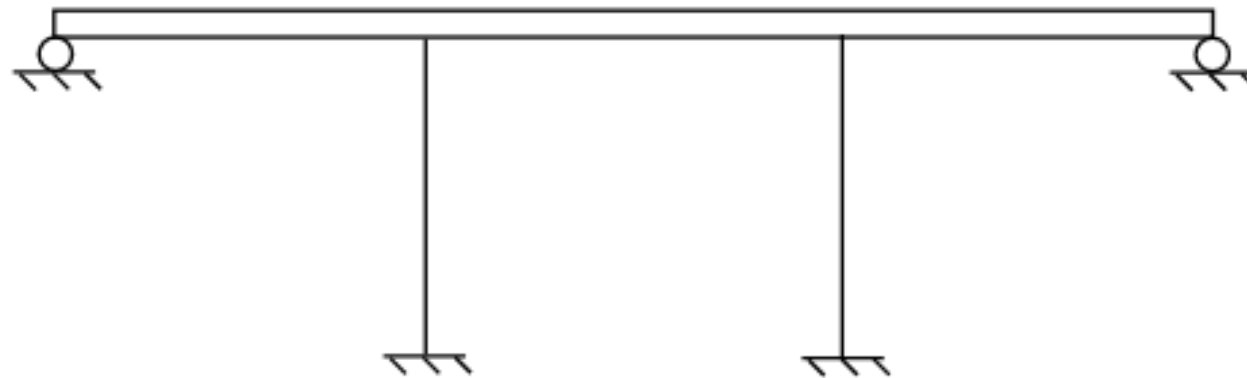


# 4 Design Details



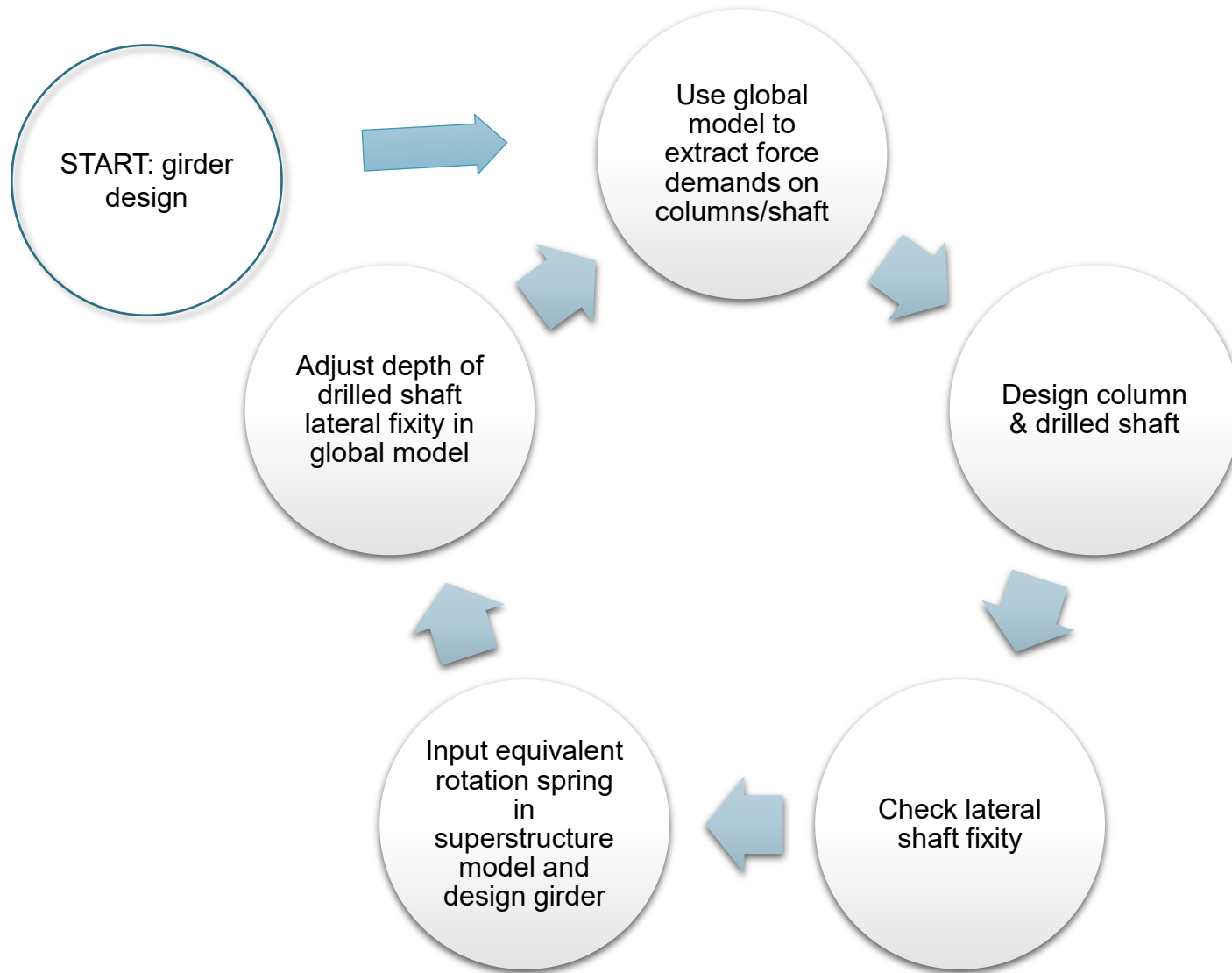
# Superstructure Model

- Typical assumption for a 3-span continuous beam

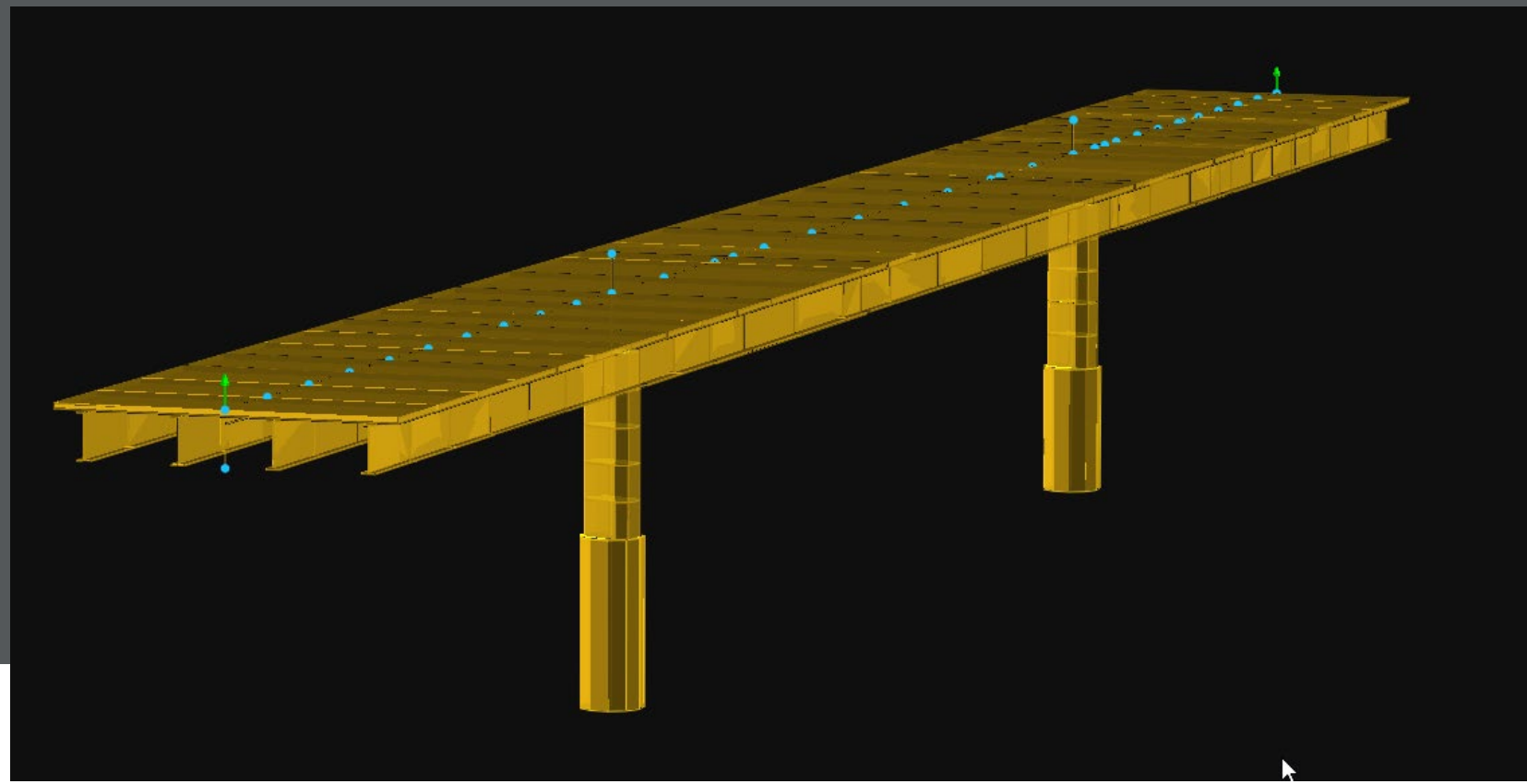




# Design Process







## Global Modeling

Spline model using beam elements

Drilled shaft fixity

Iterate on column/shaft interface lateral deflection



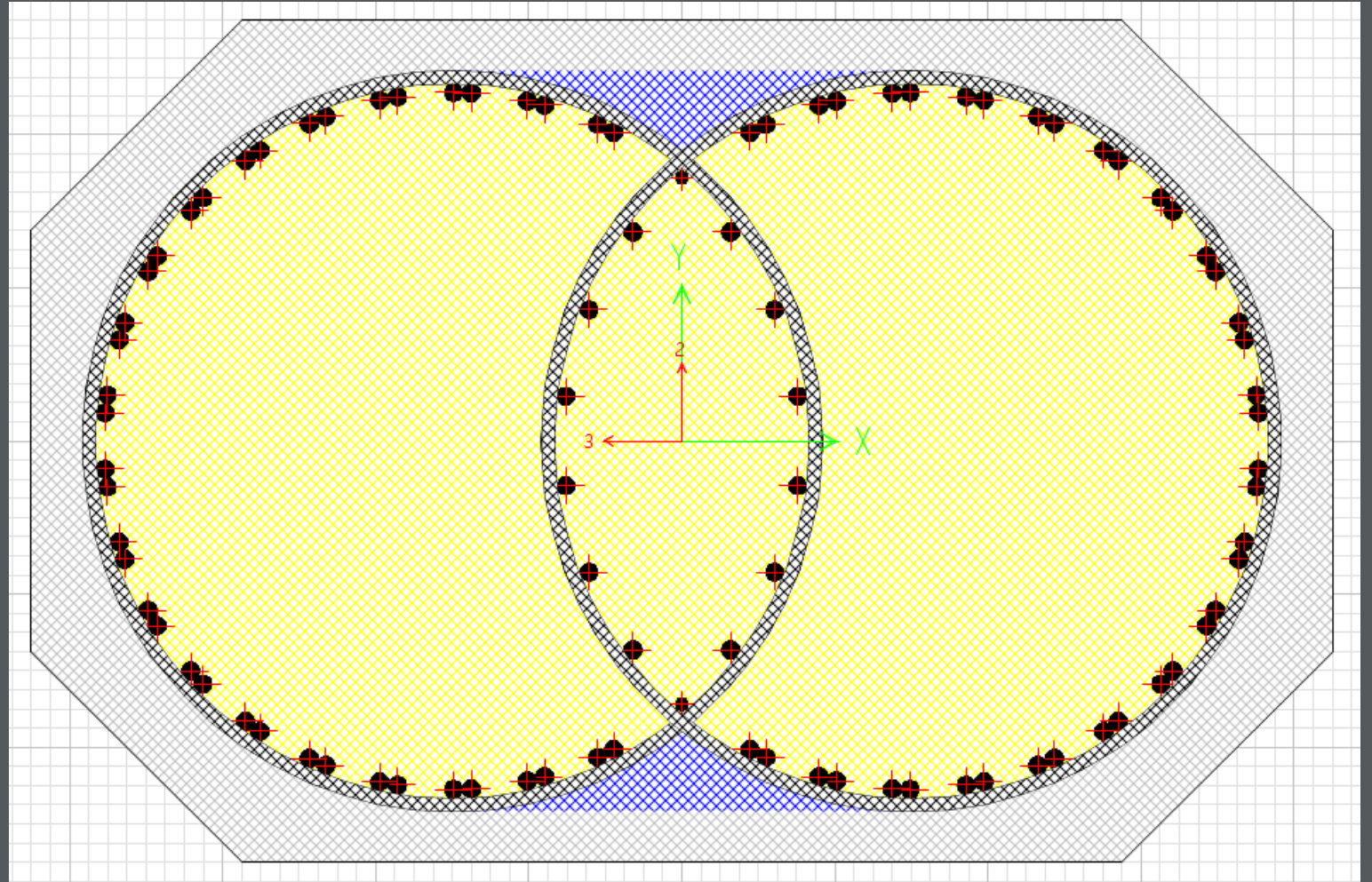
		Pier 1									
				NB			SB				
Iteration #	Shaft Dia. (ft)	Starting DS L(ft)	D	LARSA (in)	Lpile (in)	% diff	Lpile (in)	% diff	Ending DS L (ft)	Ending D	
1	9.84	27.46	2.8	0.566	0.440	22%	0.426	25%	24.57	2.5D	
2	9.84	24.57	2.5	0.508	0.603	-19%			27.11	2.8D	
3	9.84	27.11	2.8	0.624	0.500	20%			25.82	2.6D	
4	9.84	25.82	2.6	0.538	0.565	-5%			26.15	2.7D	
5	9.84	25.60	2.6	0.483	0.412	15%			24.69	2.5D	
6	9.84	24.69	2.5	0.457	0.472	-3%			24.90	2.5D	
7	9.84	24.69	2.5	0.462	0.472	-2%			24.83	2.5D	
8	9.84	24.69	2.5	0.468	0.479	-2%			24.84	2.5D	
9	9.84	24.69	2.5	0.494	0.732	-48%			28.07	2.9D	
10	9.84	28.07	2.9	0.630	0.559	11%			27.30	2.8D	
11	9.84	27.30	2.8	0.567	0.473	17%			26.22	2.7D	
12	9.84	25.98	2.6	0.548	0.565	-3%	0.556	-1%			

**Iterations with the modeling**

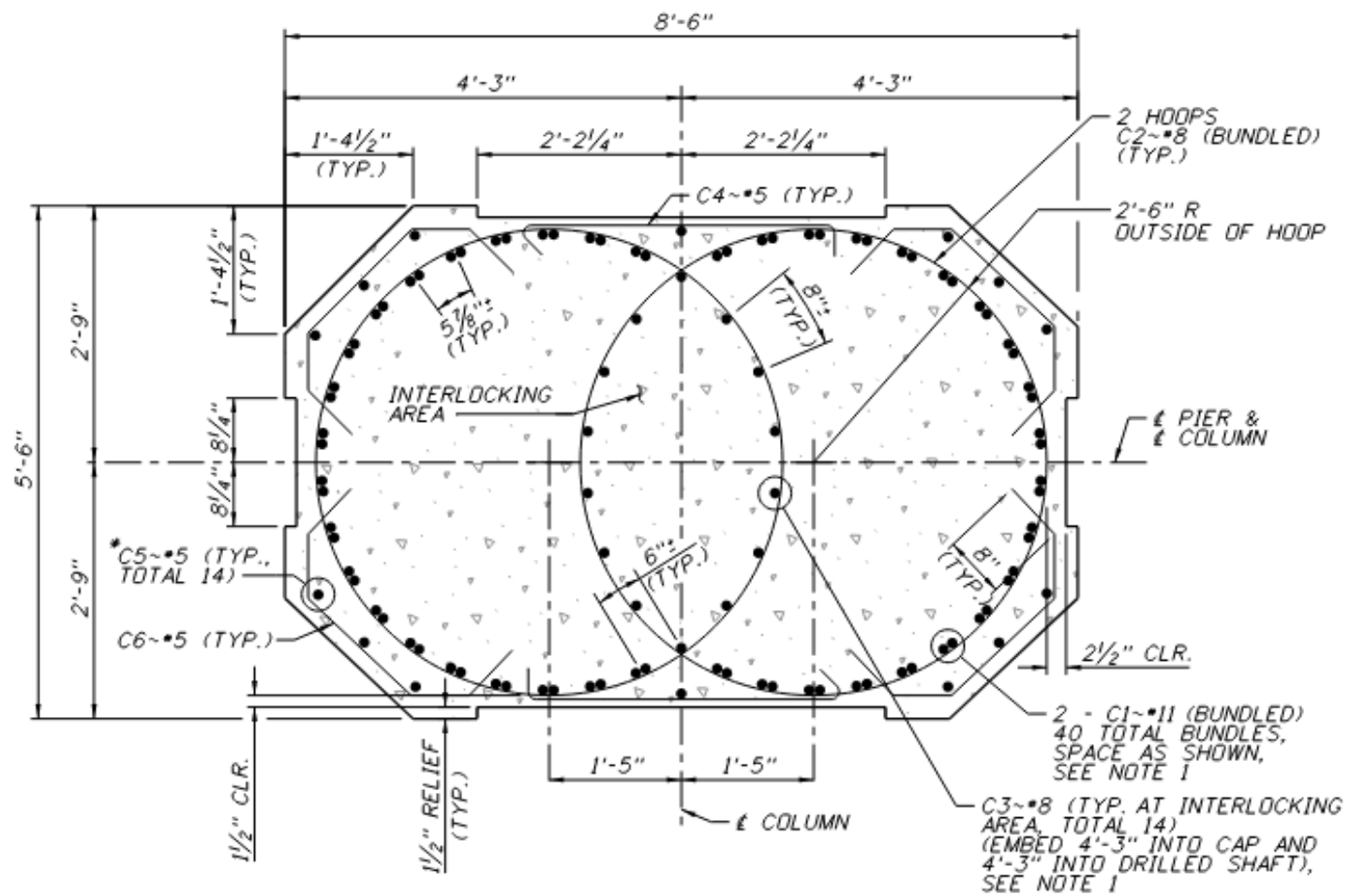


# Column Section

- 8.5ft x 5.5ft
- $f'_c = 5000$  psi
- Bundled #11's vertical steel
- Bundled #8 hoops
- 2 interlocking hoops
- 28ft tall

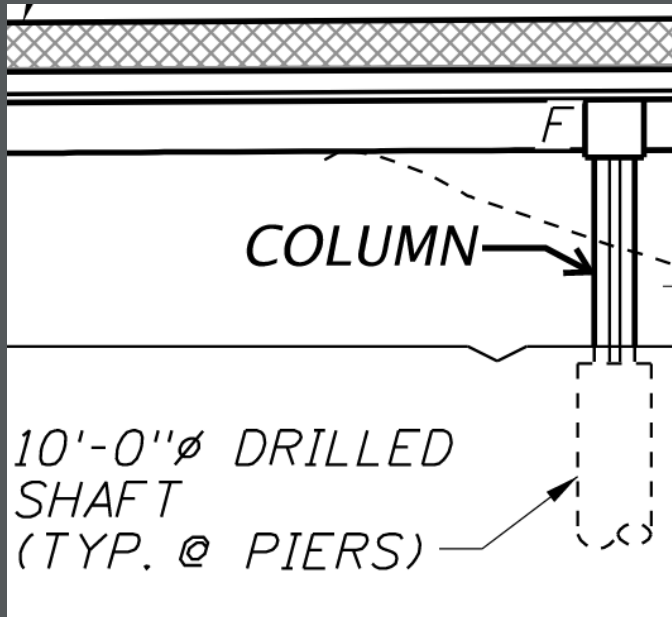




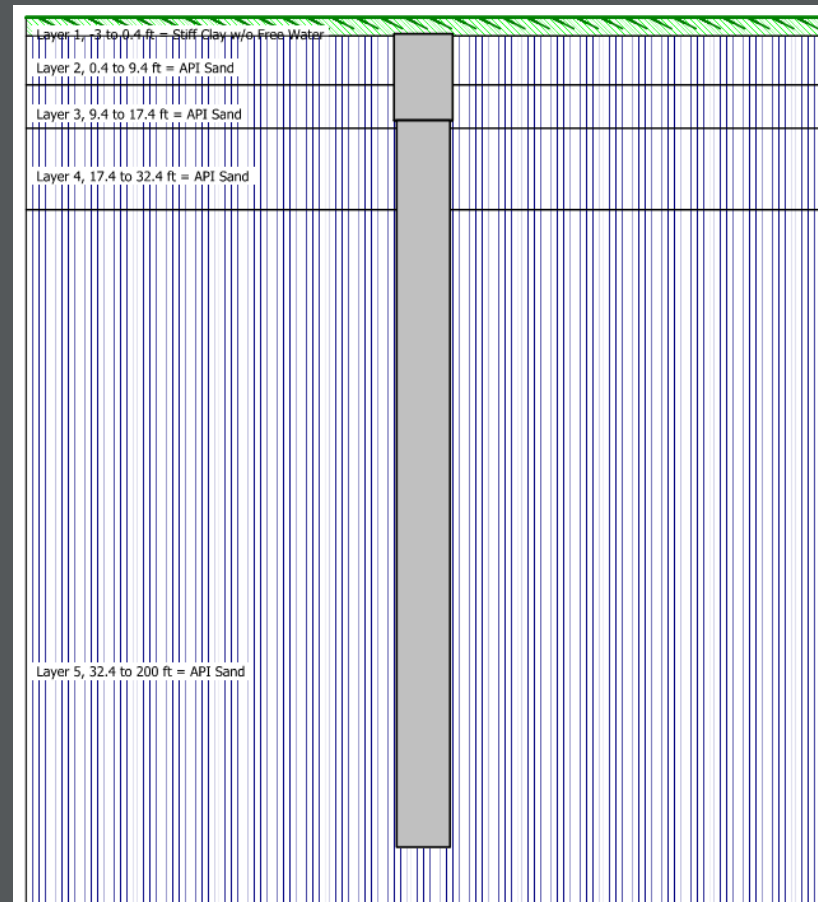


# Column Section

# Drilled Shaft Foundation



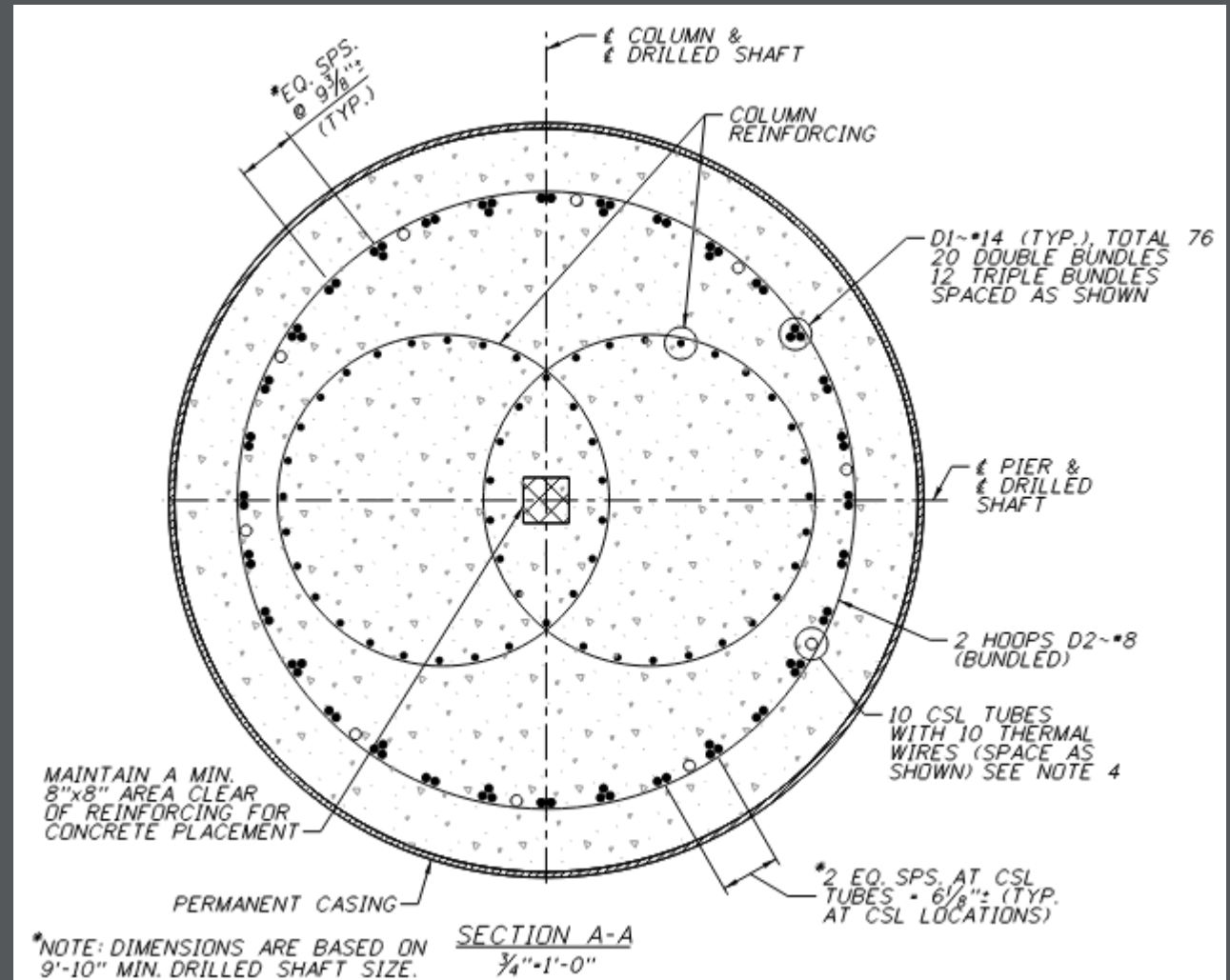
- Column frames into drilled shaft foundation
- ~111ft shaft length





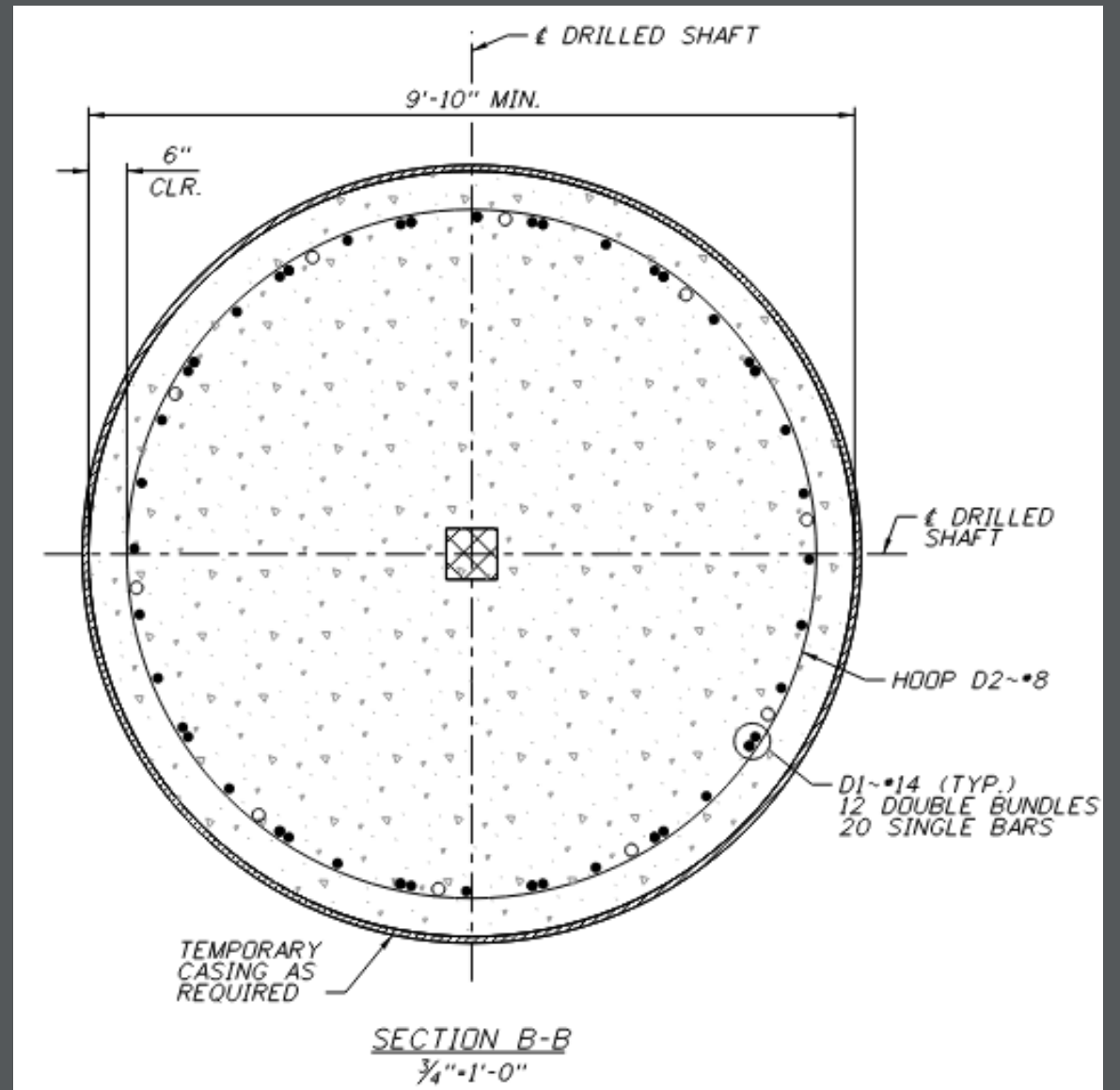
# Drilled Shaft Section

- 3.0m diameter (9.8ft)
- $f'_c = 4000$  psi
- Grade 80 reinforcement
- Upper Section
  - 20 Triple Bundles
  - 12 Double Bundles
  - #14's - vertical
  - Bundled #8 hoops

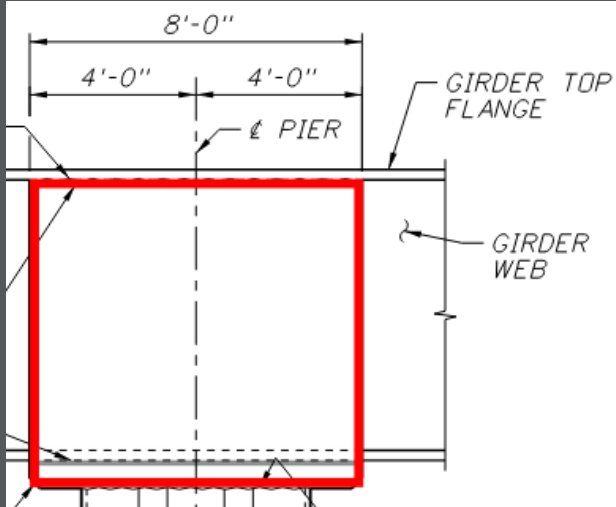
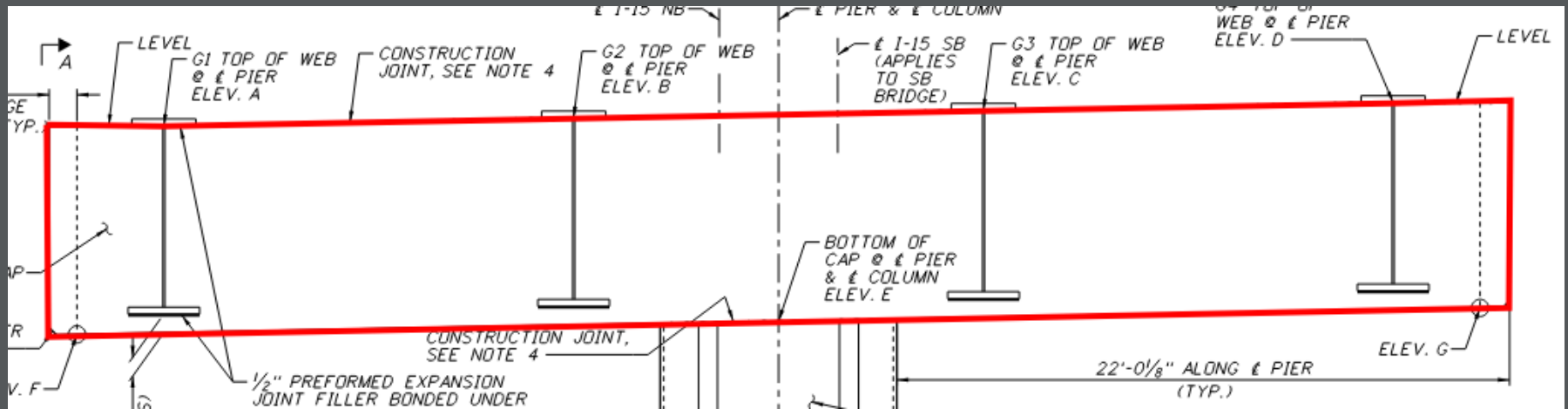


# Drilled Shaft Section

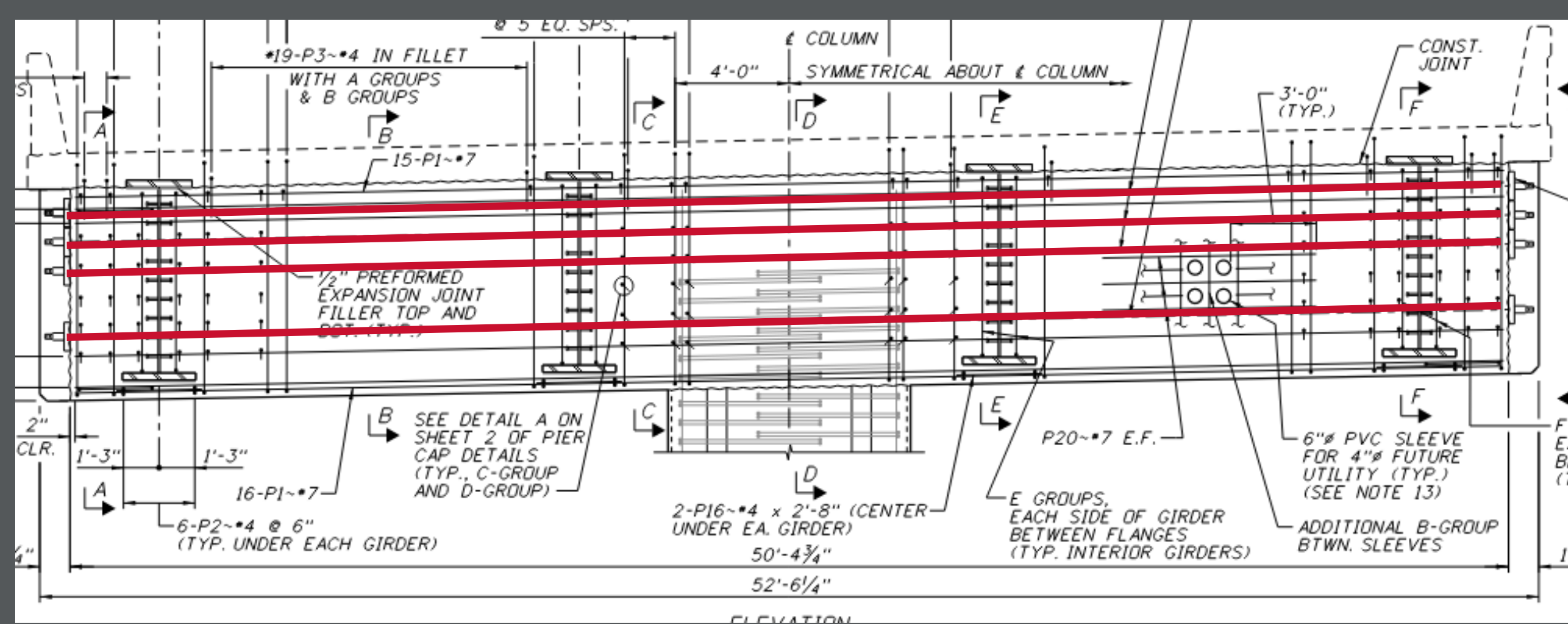
- Lower Section
  - 20 Double Bundles
  - 12 Single bars
  - #14's - vertical
  - Single #8 hoops







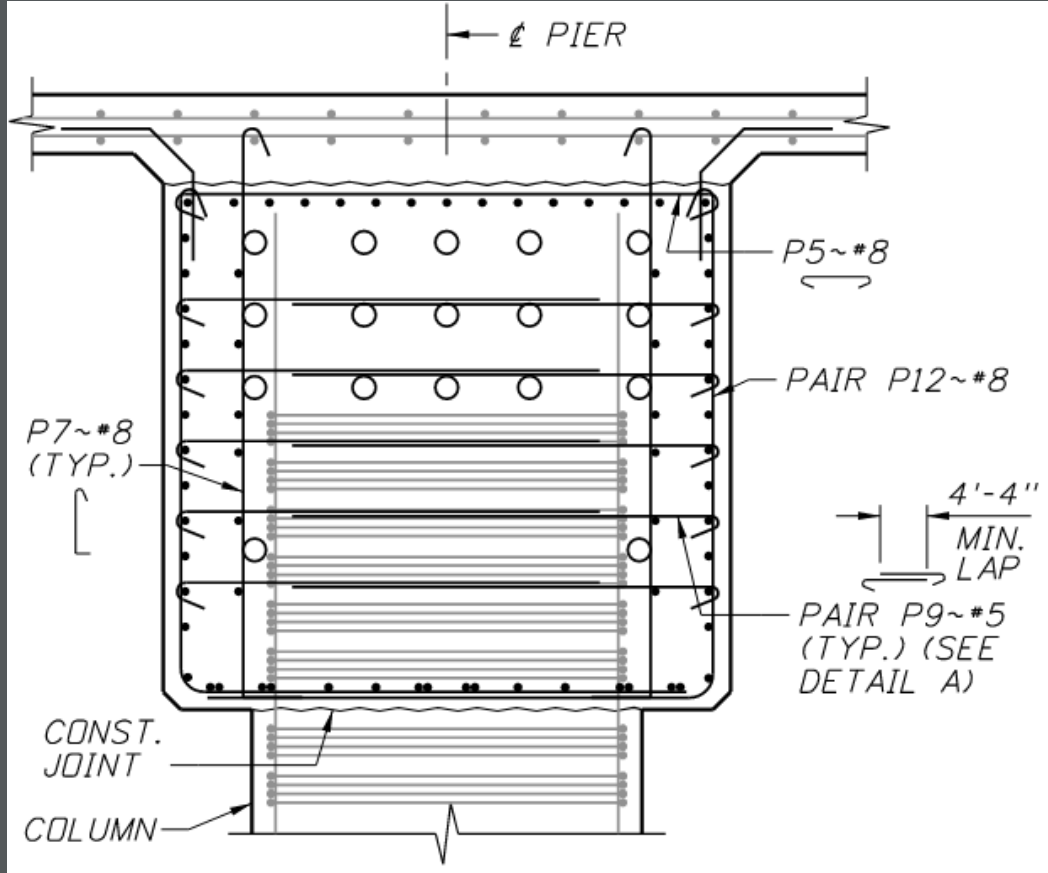
# Integral Pier Cap



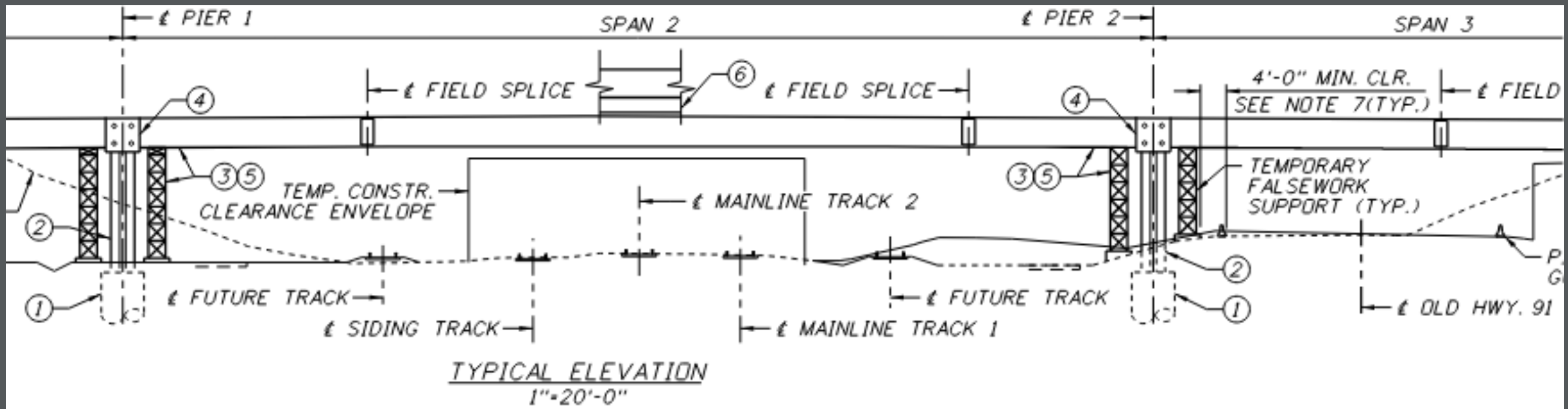
# Integral Pier Cap



# Integral Pier Cap Details



- $f'c = 6000$  psi
- 17 High strength post tensioning bars
  - $f_{pu} = 150$ ksi
  - 2 1/2" diameter
- Bar jacking force of 542 kips, or 542,000 lbs
- #8 transverse bars for shear and torsion resistance



## Construction Sequence

1. Foundation
2. Columns
3. Erect falsework supports
4. Place girders on temp supports
5. Construct pier cap



# Challenges Revisited

## How were they addressed?

Skew of railroad tracks	Reduced skew by using single column piers
Vertical clearance of tracks	Integral caps; Raise I-15 grade ~8ft; limit superstructure depth
UPRR ROW	Abut 1 and Pier 1 located in ROW; Pier 2 located outside of ROW
Proximity of piers to railroad tracks	Locate piers >27ft from future siding tracks; no crash walls
Utilities	Bury overheads
MOT during construction	Crossovers on I-15; temporary closures on Old 91

# 05

## Lessons Learned

- Check with specialty contractors on specialty design items – they can provide excellent feedback.
- Seek guidance from other governing agencies when client doesn't have experience/guidance.
- Large member sizes = large equipment; be aware of space



HDR

**Q & A**

**HR**